

Rainfall-Runoff Hydrograph Prediction Using a Dynamic Wave Based Instantaneous Unit Hydrograph

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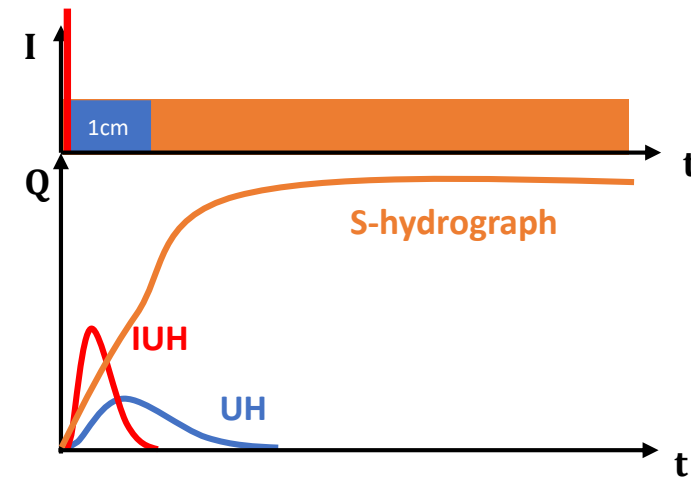


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- II. Dynamic wave based IUH (DIUH) method
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Concept of Unit hydrograph (UH)

- ✓ **Unit hydrograph (UH)** is a **direct runoff hydrograph** resulting from **one unit (1 cm or 1 inch) of effective rainfall** generated uniformly over the entire watershed at a constant rate for an effective duration.
- ✓ A UH model is a simple **linear model** that can be used to derive **runoff hydrographs** using the principles of superposition and proportionality.
- ✓ The UHs for a watershed have traditionally been derived by relating **measured rainfall and runoff discharge**.
- ✓ If the effective rainfall is of unit amount and its **duration is infinitesimally small**, the unit hydrograph may be called as an **Instantaneous unit hydrograph (IUH)**.
- ✓ **S-hydrograph** is a direct runoff hydrograph generated by a **continuous effective rainfall** occurring at an uniform rate for an indefinite period.
- ✓ S-hydrograph can be obtained by **summation of an infinite series of UHs or IUHs**.



Linsley, R. K., Kohler, M. A., Paulhus, J. L. H., 1958. Hydrology for Engineers. McGraw-Hill, New York.

Chow, V., 1964. Applied Hydrology. McGraw-Hill, New York.

Concept of Unit hydrograph (UH)

✓ Brief history of UH models

- Sherman (1932) : Introduction of unit hydrograph (UH)

Sherman, L.K., 1932. Streamflow from Rainfall by Unit-Graph Method. Engineering News-Record. 108, 501-505.

- Snyder (1938), Clark (1945) : Synthetic Unit hydrographs

Snyder, F. F., 1938. Synthetic Unit Graphs. Eos, Transactions American Geophysics Union. 19, 447-454.

Clark, C.O., 1945. Storage and the Unit Hydrograph. Transactions of the American Society of Civil Engineers. 110 (1), 1419-1446.

- Nash (1957), Dooge (1959) : Instantaneous unit hydrographs (IUH)

Nash, J.E., 1957. The Form of the Instantaneous Unit Hydrograph. International Association of Hydrological Sciences. 45, 114-121.

Dooge, J.C.I., 1959. A general theory of the unit hydrograph. Journal of Geophysical Research. 64 (2), 241-256.

- Rodriguez-Iturbe and Valdes (1979), Gupta et al. (1980) : Geomorphological instantaneous unit hydrographs (GIUH)

Rodriguez-Iturbe, I., Valdes, J.B., 1979. The geomorphologic structure of hydrologic response. Water Resources Research. 15(6), 1409-1420.

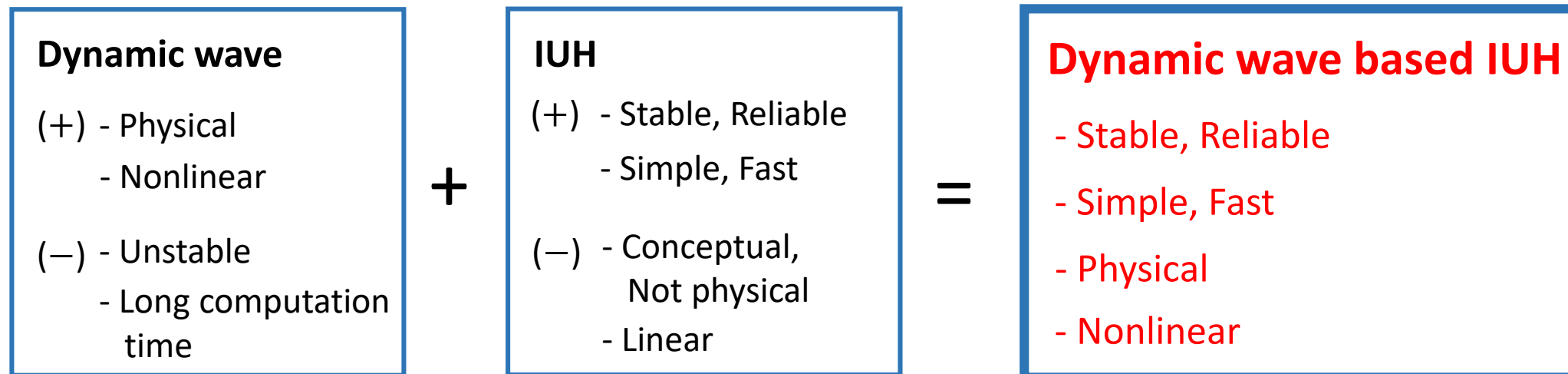
Gupta, V.K., Waymire, E., Wang, C.T., 1980. A representation of an instantaneous unit hydrograph from geomorphology. Water Resources Research, 16(6), 855-862.

- Lee and Yen (1997) : Kinematic wave based GIUH (KW-GIUH)

Lee, K. T., Yen, B. C., 1997. Geomorphologic and kinematic-wave-based hydrograph derivation. Journal of Hydraulic Engineering. 123(1), 73– 80.

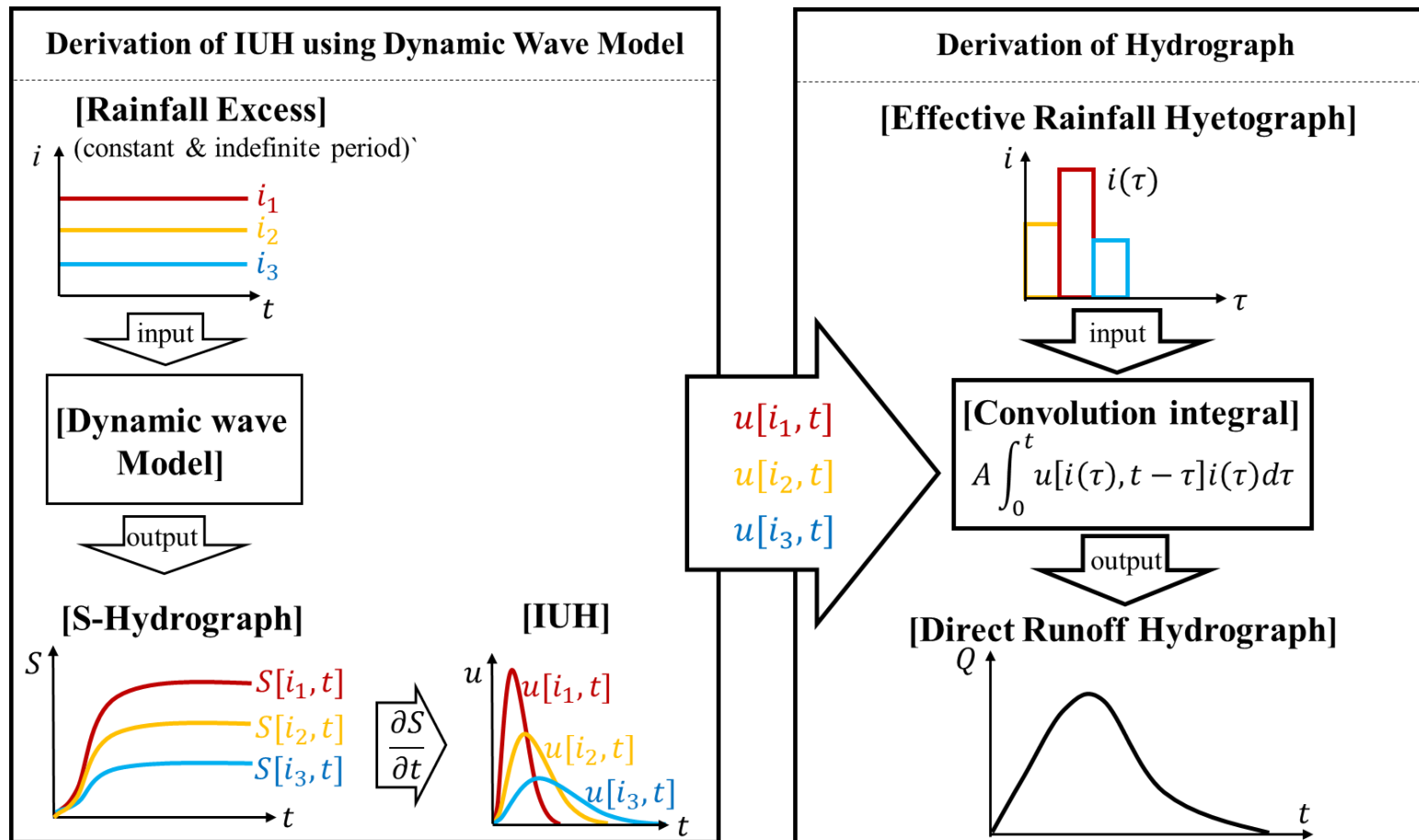
Dynamic wave based IUH (DIUH) method

- ✓ **Dynamic wave models** are known as one of the most **physically representative and accurate rainfall-runoff** modeling methods, thus can consider the **nonlinear relationship between rainfall and runoff**.
- ✓ However, the dynamic wave models are not appropriate for a practical purpose owing to their **numerical instability and high computational cost**.
- ✓ Our purpose is to present a **hybrid method composed of a dynamic wave approach and instantaneous unit hydrograph (IUH)** to predict rainfall-runoff discharge physically, stably, and instantly.



Dynamic wave based IUH (DIUH) method

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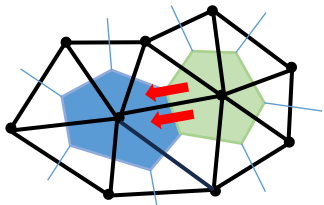
Schematic of dynamic wave based IUH (DIUH) method

Dynamic wave based IUH (DIUH) method

Step 1 : S-hydrograph Generation

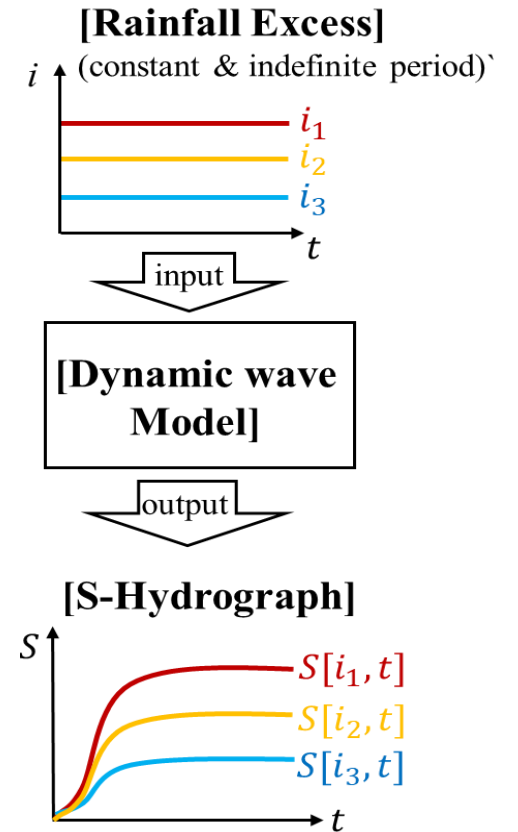
- ✓ **S-hydrographs** corresponding to each of the **various rainfall intensities** are produced by **simulating** uniform rainfalls in a watershed using a **dynamic wave model**.
- ✓ The **dynamic wave model** we use is a fully coupled hydrologic and hydrodynamic model, **tRIBS-FEaST** developed by Kim et al. (2012, 2013).
- ✓ The governing equations of FEaST for fluid motion are the **shallow water equations**.

$$\frac{\partial h}{\partial t} + \frac{\partial(Uh)}{\partial x} + \frac{\partial(Vh)}{\partial y} = S_r, \quad \frac{\partial(Uh)}{\partial t} + \frac{\partial(U^2h + \frac{1}{2}gh^2)}{\partial x} + \frac{\partial(UVh)}{\partial y} = -gh \frac{\partial z_b}{\partial x} - C_D U \sqrt{U^2 + V^2},$$



$$\frac{\partial(Vh)}{\partial t} + \frac{\partial(UVh)}{\partial x} + \frac{\partial(V^2h + \frac{1}{2}gh^2)}{\partial y} = -gh \frac{\partial z_b}{\partial y} - C_D V \sqrt{U^2 + V^2}$$

- ✓ To solve the governing equations, **the finite volume method** on an unstructured triangular grid and **Roe's approximate Riemann solver** (Roe, 1981) is used.



Kim, J., Warnock, A., Ivanov, V.Y., Katopodes, N.D., 2012. Coupled modeling of hydrologic and hydrodynamic processes including overland and channel flow. *Advances in Water Resources*. 37, 104–126.

Kim, J., Ivanov, V.Y., Katopodes, N.D., 2013. Modeling erosion and sedimentation coupled with hydrological and overland flow processes at the watershed scale. *Water Resources Research*. 49, 5134-5154.

Roe, P.L., 1981. Approximate Riemann solvers, parameter vectors, and difference schemes. *Journal of Computational Physics*. 43, 357-372.

Dynamic wave based IUH (DIUH) method

Step 2 : Derivation of Dynamic wave based IUH

- ✓ Mathematically, **the IUH is the first derivative of an S-hydrograph** normalized by the rainfall excess intensity and the area of a watershed.

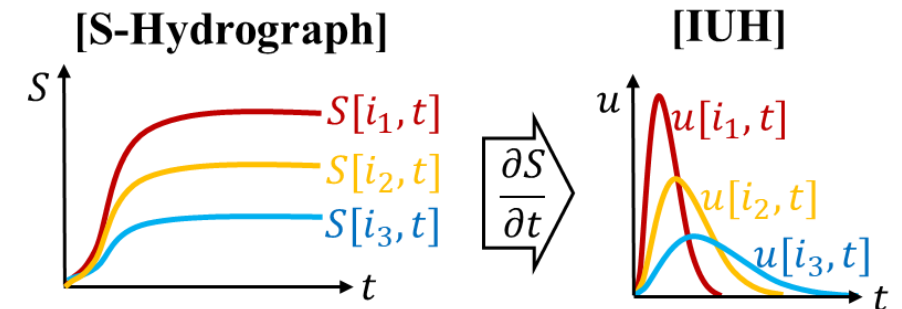
$$u(t) = \frac{1}{i_o A} \frac{dS(t)}{dt}$$

- ✓ Because $S(t)$ is computed numerically with the dynamic wave model, we calculate $u(t)$ using a discretized equation.

$$u(t) = \frac{S(t + 0.5\Delta t) - S(t - 0.5\Delta t)}{i_o A \Delta t} + O(\Delta t^2)$$

- ✓ Since we produced S-hydrographs corresponding to each of the various rainfall intensities, **the DIUH we derived is a function of both the watershed and rainfall properties.**

Ding, J. Y., 2011. A measure of watershed nonlinearity: interpreting a variable instantaneous unit hydrograph model on two vastly different sized watersheds. Hydrology and Earth System Sciences. 15(1), 405–423.



Dynamic wave based IUH (DIUH) method

Step 3 : Hydrograph Derivation

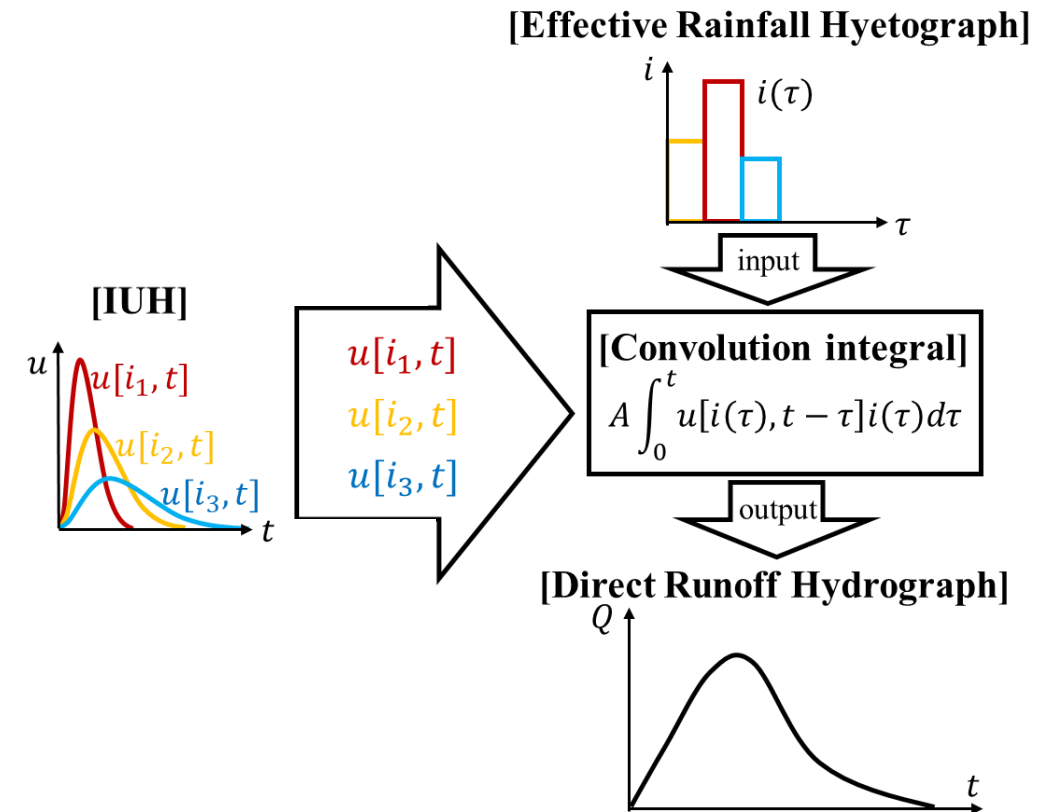
- ✓ To derive **the direct surface runoff hydrograph** of a watershed, we use an **alternative form of convolution integral proposed by Amorocho (1967)**.

$$Q(t) = A \int_0^t u[i(\tau), t - \tau] i(\tau) d\tau$$

- ✓ Because our method produces only discrete values of the DIUH, we numerically integrate the convolution integral equation.

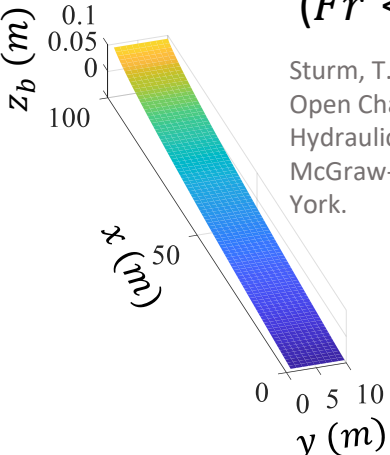
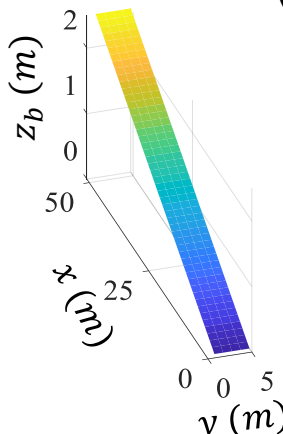
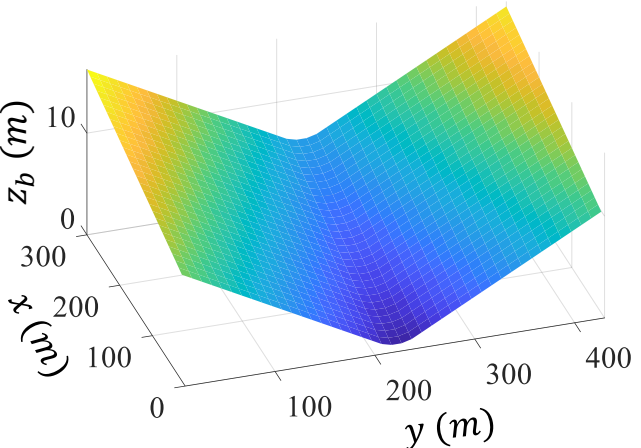
$$Q(t) = A \sum_{\tau=0}^t u[i(\tau), t - \tau] i(\tau) \Delta\tau$$

- ✓ The DIUH in the equation, $u[i(\tau), t - \tau]$, is a function of both time and rainfall intensity for a watershed, thus **nonlinear relationship between rainfall and runoff can be considered**.



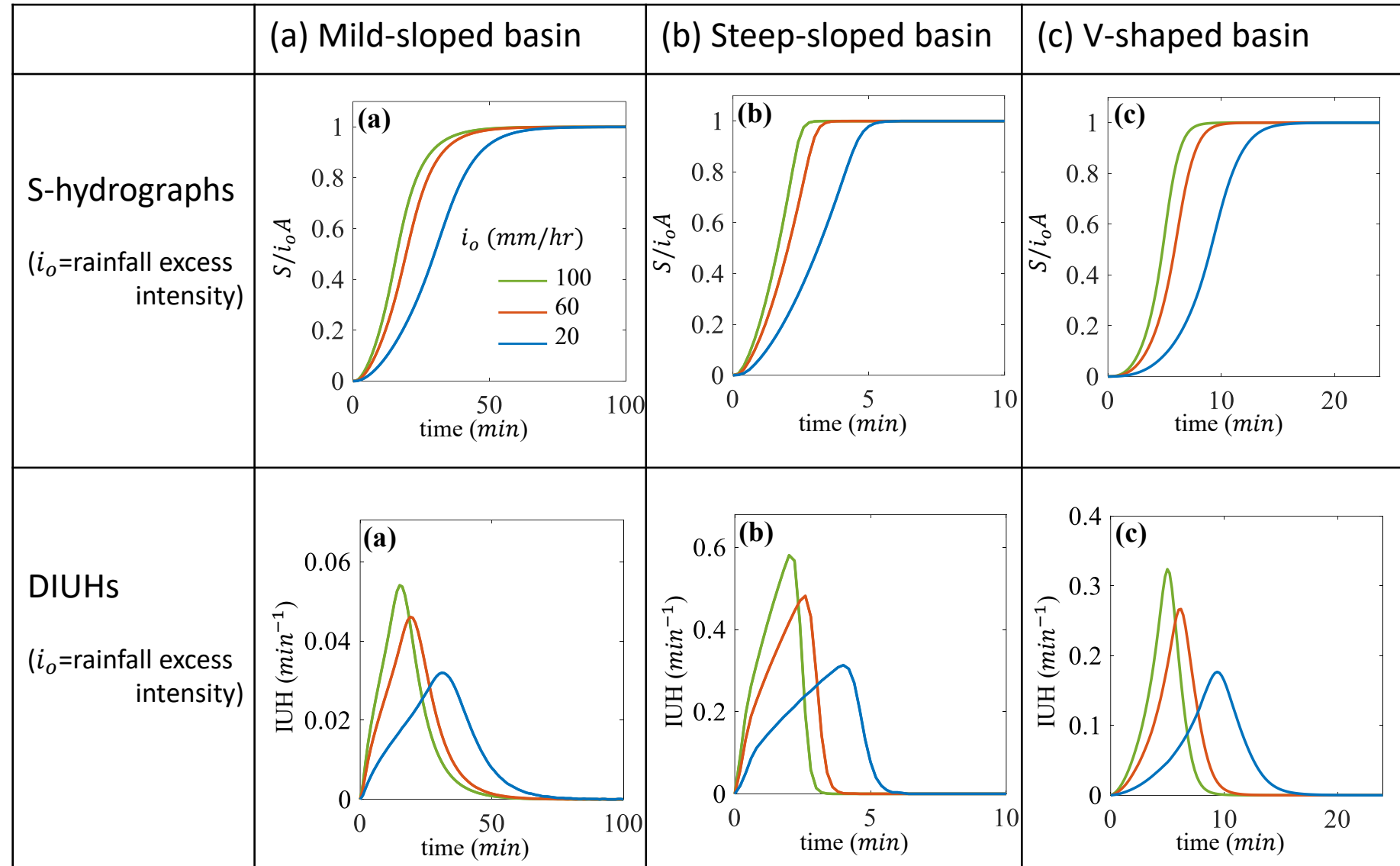
Test in ideal basins

✓ To test the performance of the proposed DIUH method, **three impervious ideal basins** are adopted.

Basin	(a) Mild-sloped basin $(Fr < 1)$  <small>Sturm, T.W., 2001. Open Channel Hydraulics. McGraw-Hill, New York.</small>	(b) Steep-sloped basin $(Fr > 1)$ 	(c) V-shaped basin 
Size	100 m × 10 m	50 m × 5 m	Two Planes: 200 m × 300 m Channel: 20 m × 300 m
Bottom slope	$S_o \propto A_d^{-\theta}$ A_d : drainage area along the main channel $\theta = 0.6$: concavity index <small>Flint, J.J., 1974. Stream gradient as a function of order, magnitude, and discharge. Water Resources Research. 10, 969– 973.</small>	0.05	Two planes : 0.05 channel : 0.02
Manning's roughness	$n = 0.04 \text{ s/m}^{1/3}$	$n = 0.015 \text{ s/m}^{1/3}$	$n = 0.015 \text{ s/m}^{1/3}$
Nodes and grids	1,224 nodes and 2,222 grids	364 nodes and 610 grids	1,408 nodes and 2,664 grids

Test in ideal basins

- ✓ Figures show S-hydrographs and **DIUHs** of the three ideal basins. (DIUHs for any i_o values can be derived.)
- ✓ The **peak** and **time to the peak** of the DIUHs are **proportional** and **inversely proportional** to the **rainfall intensity**, respectively.
- ✓ Therefore, DIUH is a **function of both the watershed and rainfall properties**.

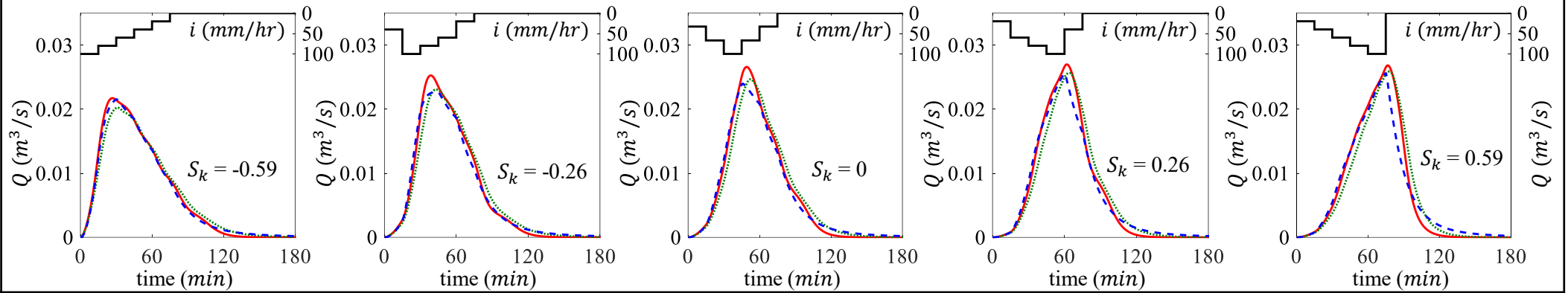


Test in ideal basins

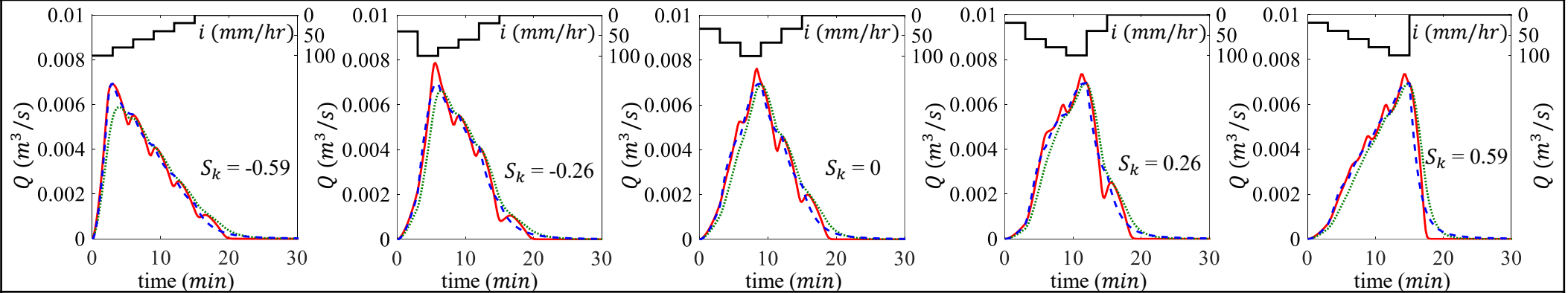
Direct surface runoff hydrographs

(a) Mild-sloped basin

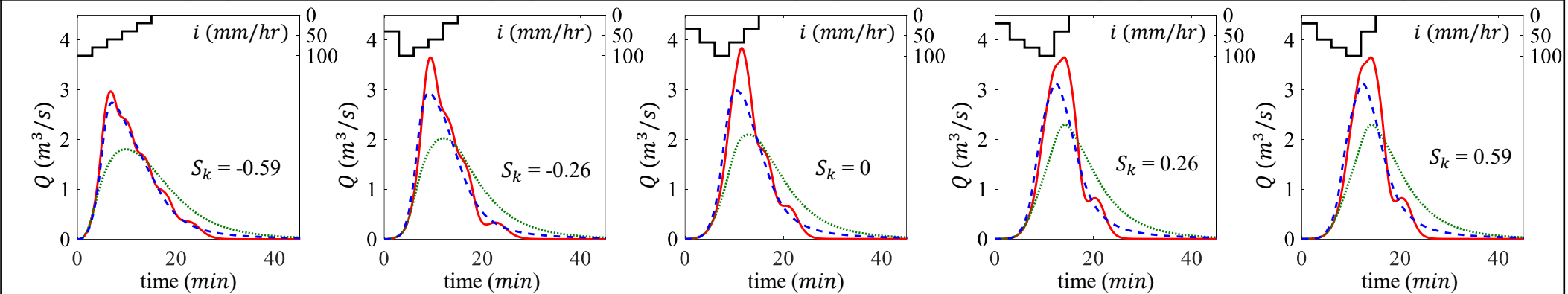
— Rainfall
— Present study
- - - KW-GIUH method
- - - Dynamic wave model



(b) Steep-sloped basin



(c) V-shaped basin



Skewness (S_k) of a rainfall hyetograph ($i(t)$)

Test in ideal basins

- ✓ We produced the **hydrographs by numerically integrating the derived DIUHs**, as shown on the previous page.
- ✓ For the temporally varying rainfall scenarios in a basins, the average of the rainfall intensities was 60 mm/h . The kurtosis of the **rainfall hyetographs** are the same, but the **skewness (S_k)** ranges from -0.59 to 0.59 .
- ✓ The hydrographs obtained with the **DIUH method show reasonable agreement with the criterion discharges** computed by the dynamic wave model, tRIBS-FEaST.

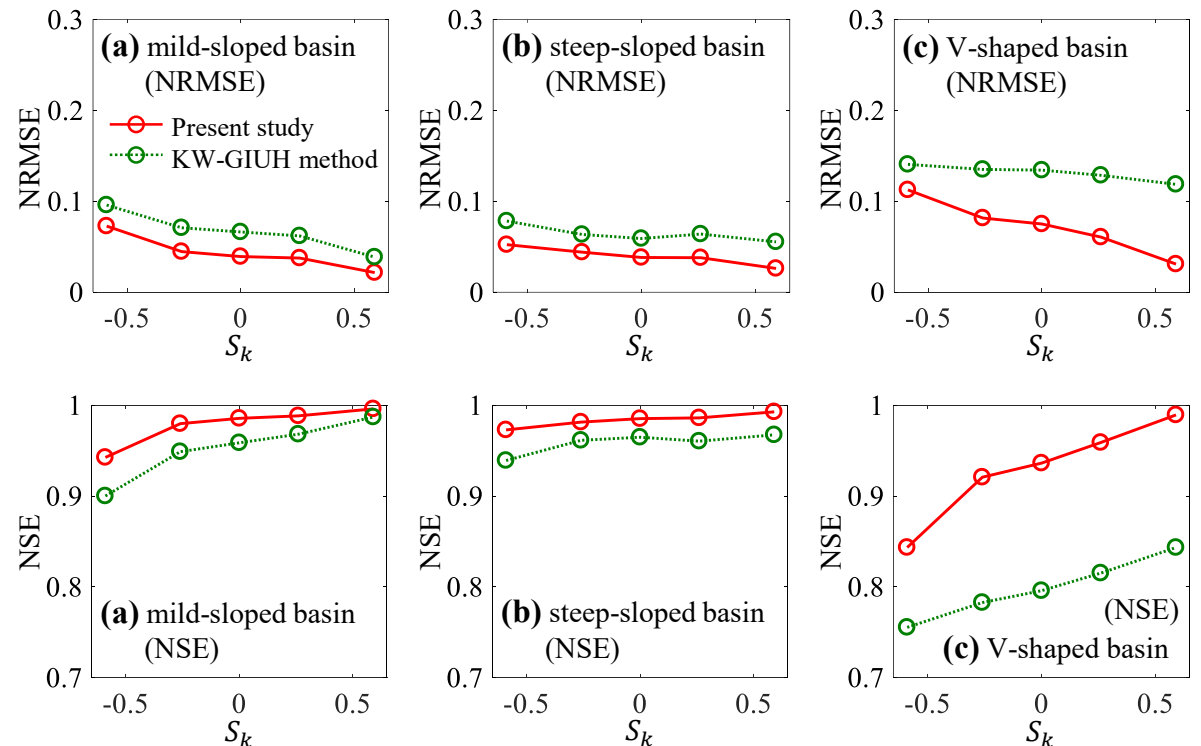
- ✓ All of the NSE for the predicted **results are in the 'very good ($NSE > 0.8$)' range.**

Nash, J.E., Sutcliffe, J. V., 1970. River flow forecasting through conceptual models part I - A discussion of principles. *Journal of Hydrology*. 10, 282-290.

Moriasi, D.N., Gitau, M.W., Pai, N., Daggupati, P., 2015. Hydrologic and water quality models: Performance measures and evaluation criteria. *Transactions of the American Society of Agricultural and Biological Engineers*. 58(6), 1763-1785.

- ✓ The **accuracy of the DIUH method increases as the S_k of the rainfall distribution increases.**
- ✓ We also compared the results with the hydrographs produced using the kinematic wave based GIUH (KW-GIUH) proposed by Lee and Yen (1997).

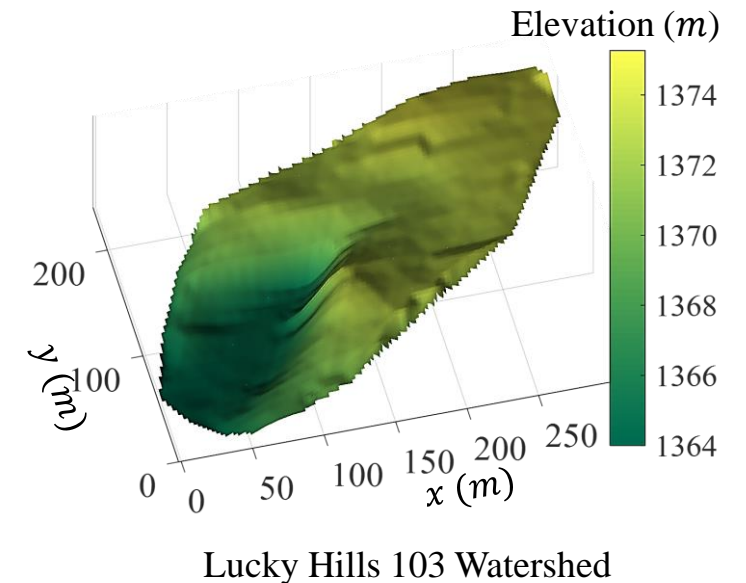
Lee, K. T., Yen, B. C., 1997. Geomorphologic and kinematic-wave-based hydrograph derivation. *Journal of Hydraulic Engineering*. 123(1), 73– 80.



Application to a real watershed

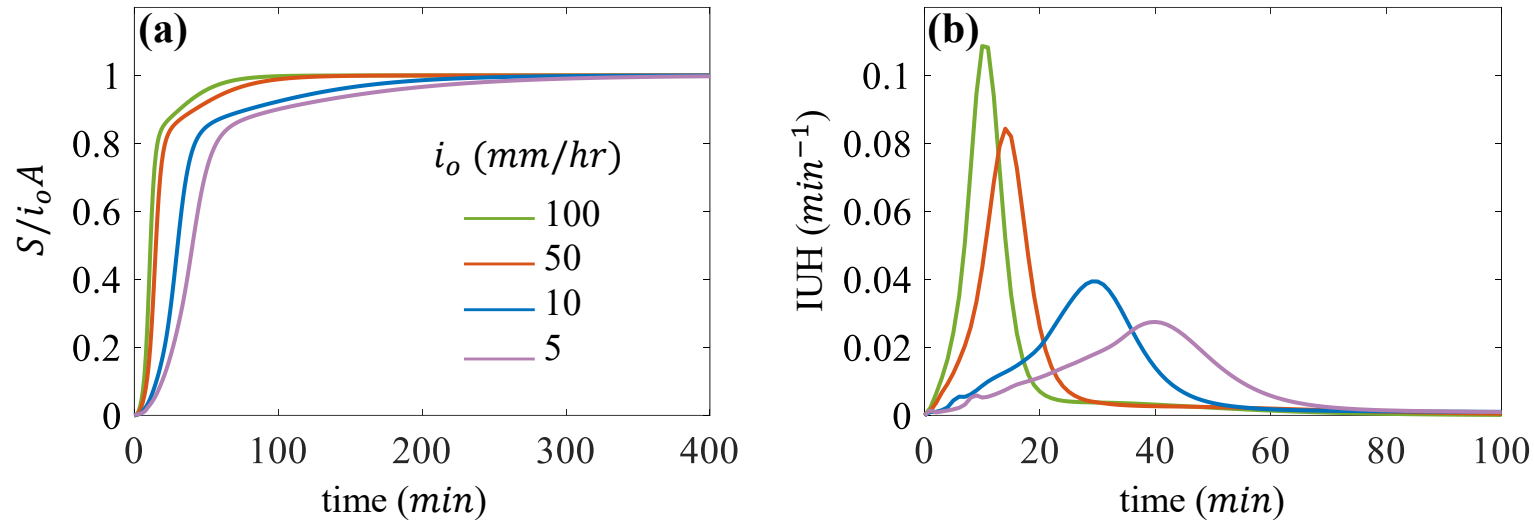
- ✓ To demonstrate the capability of the proposed DIUH method for runoff prediction in a real watershed, we applied the proposed method to a small-size experimental watershed, the **Lucky Hills 103 Watershed**.

Lucky Hills 103 Watershed	
Location	Walnut Gulch Experimental Watershed, southeastern Arizona, USA
Area	36,800 m ²
Elevation	ranges from 1,364 to 1,375 m above sea level
Average slope	approximately 0.03
Nodes and grids	481 nodes and 928 grids



Application to a real watershed

- ✓ Figures show (a) **S-hydrographs** and (b) **DIUHs** for the **Lucky Hills 103 Watershed**.

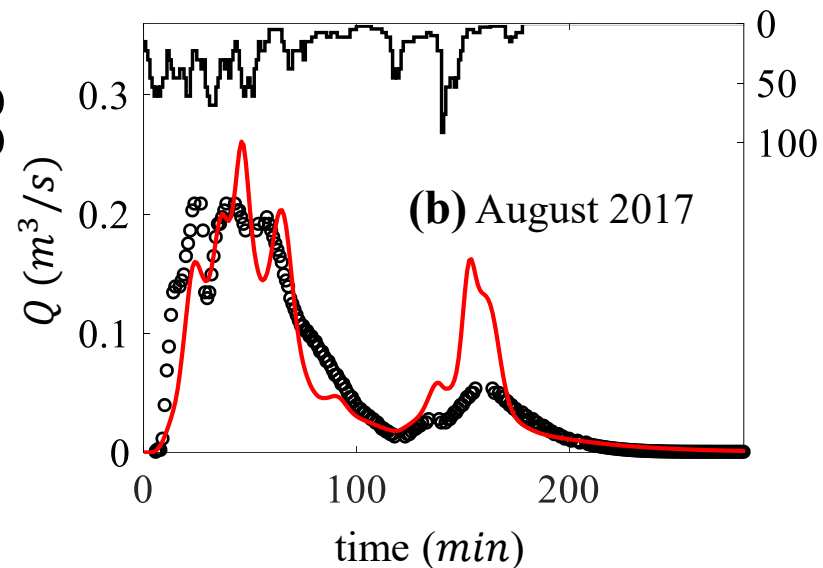
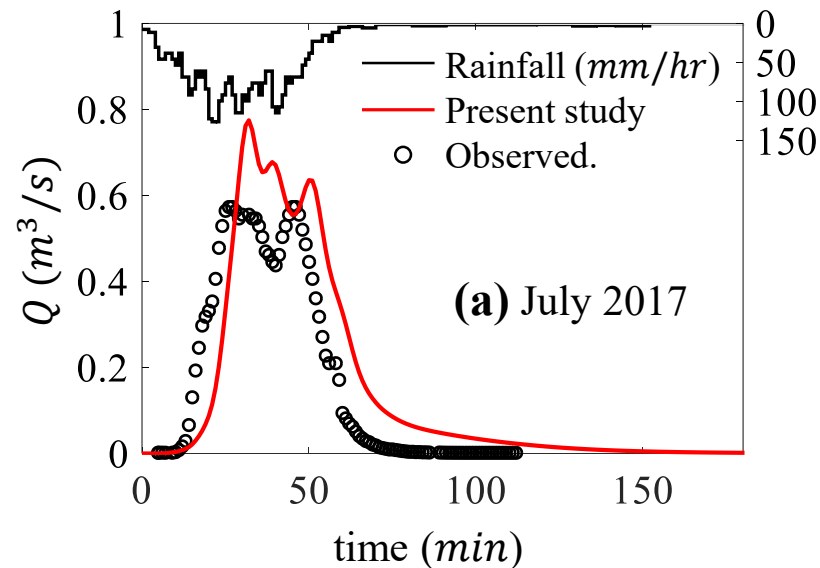


- ✓ **DIUHs** in the Lucky Hills 103 watershed showed **consistent characteristics with those in ideal basins**.

- The **peak** and **time to the peak** of the DIUHs are **proportional** and **inversely proportional** to the **rainfall intensity**, respectively.
- DIUH is a **function of both the watershed and rainfall properties**.

Application to a real watershed

- ✓ Figures show the measured and predicted runoff discharges of the **two rainfall-runoff events** in the Lucky Hills 103 Watershed.



- ✓ To consider the rainfall loss, the infiltration was estimated using the **Green–Ampt model** (Green and Ampt, 1911)

Green, W.H., Ampt, G.A., 1911. Studies on Soil Physics. Journal of Agricultural Science. 4(1), 1–24.

- ✓ The DIUH method resulted in NRMSE=0.232 and NSE=0.648 for event(a) and NRMSE=0.141 and NSE=0.788 for event(b).

Nash, J.E., Sutcliffe, J. V., 1970. River flow forecasting through conceptual models part I - A discussion of principles. Journal of Hydrology. 10, 282-290.

- ✓ Therefore, the proposed method can result in **'good' performance (NSE>0.6) for the real watershed.**

Moriassi, D.N., Gitau, M.W., Pai, N., Daggupati, P., 2015. Hydrologic and water quality models: Performance measures and evaluation criteria. Transactions of the American Society of Agricultural and Biological Engineers. 58(6), 1763-1785.

Conclusion

- ✓ To predict rainfall-runoff discharge physically, stably, and instantly, a method predicting hydrograph, based on the IUH approach combined with a dynamic wave model, is presented.
- ✓ The presented method is capable of generating IUHs without measured runoff data only if the bottom friction drag is known.
- ✓ The DIUH is a function of both the watershed characteristics and rainfall intensity, thus the nonlinear relationship between rainfall and runoff can be partially considered in the hydrograph derivation.
- ✓ The peak and time to peak of the DIUHs were proportional and inversely proportional to the rainfall intensity, respectively.
- ✓ The test results for several rainfall-runoff events on ideal and real watersheds show reasonable accuracy, thus we can conclude that the proposed DIUH method can provide good performance for both simple and complex basins.
- ✓ The proposed method still needs further improvement in practical applications.
 - The method should be coupled with more physical and accurate infiltration and interception models.
 - We need to resolve the inaccuracy observed in the recession limb of the hydrographs.
 - Nonlinear methods considering the relationship among the water depth, flow velocity, and celerity should be adopted to improve the prediction accuracy.

Acknowledgement

- ✓ This research was supported by Basic Science Research Program through the National Research Foundation of Korea (NRF) funded by the Ministry of Science, ICT and Future Planning (2017R1E1A1A01074399).

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- ✓ Sturm, T.W., 2001. *Open Channel Hydraulics*. McGraw-Hill, New York.

Thank you
